



UNIVERSITÀ
DEGLI STUDI
DI BRESCIA



*Department Civil, Environmental, Architectural
Engineering and Mathematics (DICATAM)*

REAL-TIME EVALUATION AND MANAGEMENT OF EXTREME TRAFFIC LOAD RISK ON MAIN ROAD'S BRIDGES

Roberto VENTURA

Benedetto BARABINO

Giulio MATERNINI



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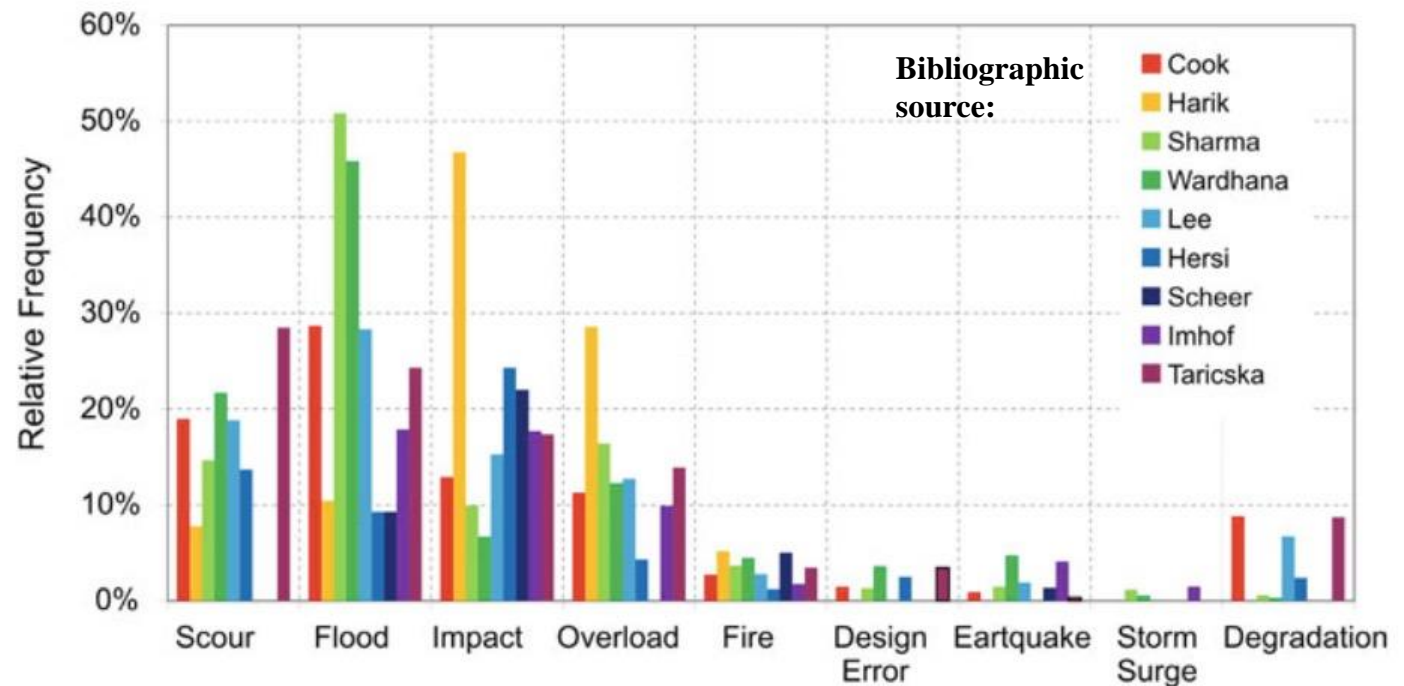
Summary

- 1. Introduction**
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- 3. Methodological Framework**
- 4. Real World Experiment**
- 5. Discussion**
- 6. Conclusions**
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1. Introduction

- **Bridges** are among the most vulnerable elements of road networks (Zanini et al, 2017).
- **Vehicular traffic hazard** (impact and overload) is one of the main causes of bridge *failures* (Proske et al, 2018).
- **Extremely heavy vehicles** above the mass limits of Traffic Codes menace bridge safety (Ventura et al, 2020).
- **Systems to real time monitor and manage the risk** of extreme traffic load are essential for bridge safety.

Source: Proske et al. (2018)



1. Introduction

The goals of this research are:

- Providing a **comprehensive list of safety factors** related to bridge failure events.
- **Real-time evaluating and managing** the **risk** induced by **extreme traffic load** on **bridges**.
- Integrating **WIM (Weight-in-Motion)** systems to collect **site specific traffic load data**.
- Developing **probabilistic models** (GLRs e ANNs) to predict the risk components (frequency and severity) by **computationally efficient** elaborations.
- Defining an **ITS (Intelligent Transportation System) architecture** based on a CD (Cloud Platform), VMS (Variable Message Signals) and TL (Traffic Lights) for a **real-time risk management strategy**.

2. Literature review

Methodologies for bridge risk assessment

Three main research directions emerged:

- I. Assumes **traffic load hazard** as a **component** of a broader **multi-hazard risk assessment framework** (e.g., Fiorillo & Nassif, 2020; Zhu & Frangopol, 2016).
- II. Focuses on the risk posed by **traffic load hazard** (e.g., Fiorillo and Ghosn, 2022).
- III. Focuses on the **consequences** induced by bridge *failure* events (e.g., Abarca et al, 2022).

Two different risk definitions emerged:

- I. States the risk as the sum of the **probability of occurrence of each *failure* scenario** times the associated **(direct and indirect) consequences** (e.g., Fiorillo and Ghosn, 2022; Cosenza and Losanno, 2021).
- II. States the risk merely as the **probability of occurrence of an undesired event** (Wijesuriya & Tennant, 2021; Davis-McDaniel et al., 2013).

2. Literature review

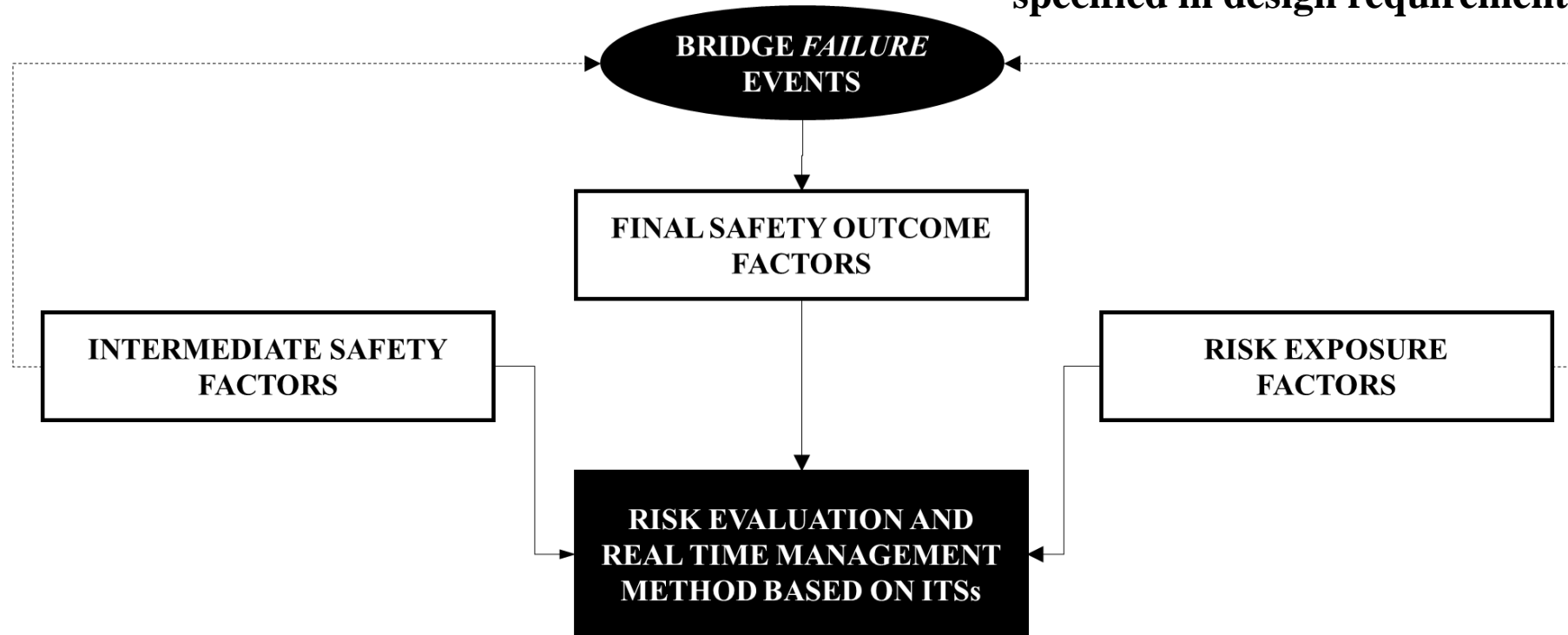
Although previous studies provided valuable results, **several gaps persists**:

- I. The absence of a comprehensive **list of safety factors** related to bridge *failure* events.
- II. The **source of analysed data** (WIM data were rarely integrated into risk analyses).
- III. The **type of predictive models** adopted to forecast the risk components (ML was never applied)
- IV. The **computational effort** required by risk management procedures (expensive techniques were proposed).
- V. The **type of proposed risk management actions** (only “static” measures were proposed).

3. Methodological framework

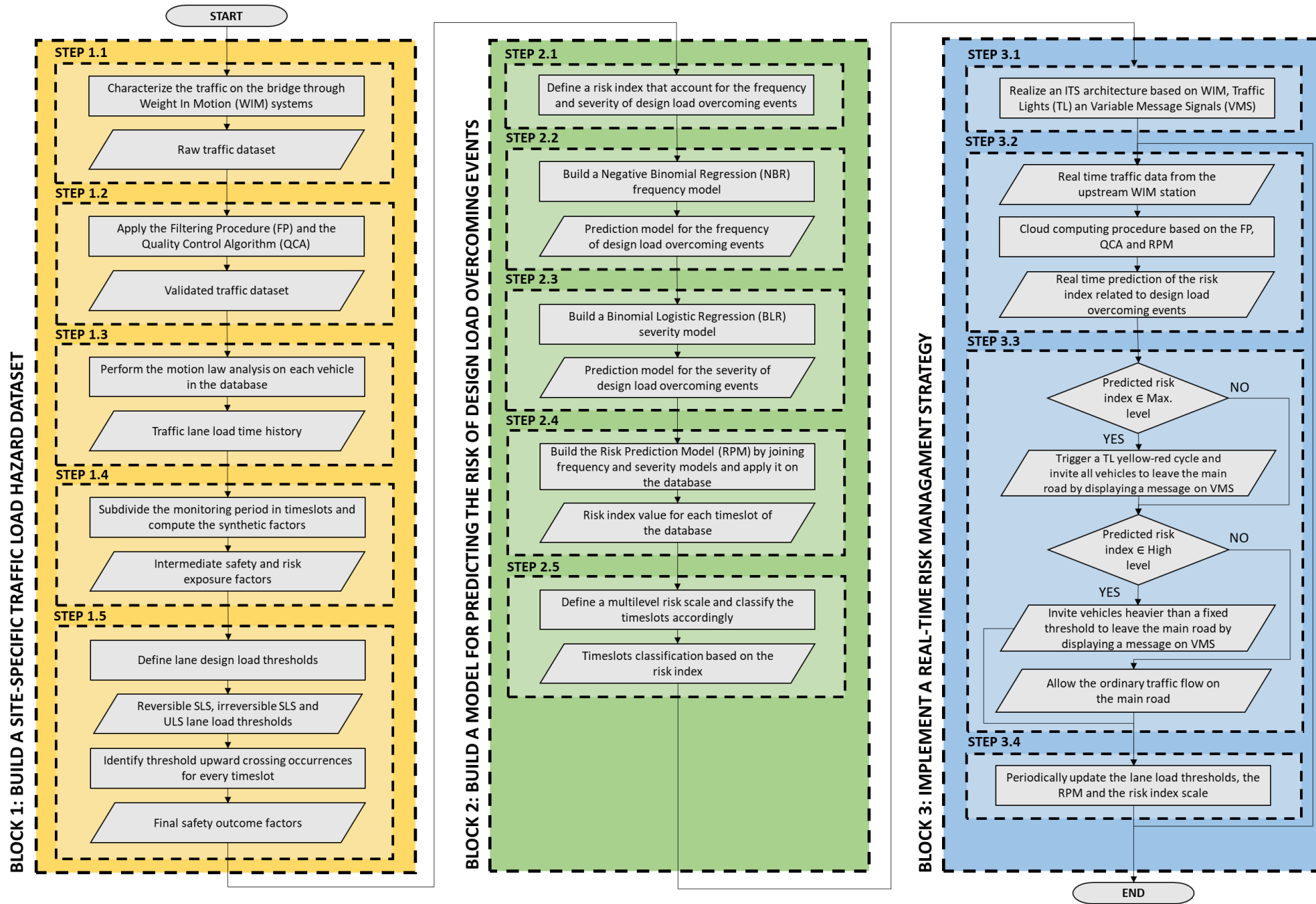
- It draws from the well-accepted framework of **ISO-EN 39001** (2012) for road safety analysis.
- It estimates the **frequency** and the **severity** of bridge *failure* events by adopting a simplified procedure inspired by the Limit State Method of **Eurocodes** (2003).

failure event $\stackrel{\text{def}}{=} \text{incapacity of performing as specified in design requirements}$



Source: Authors elaboration (2023)

3. Methodological framework

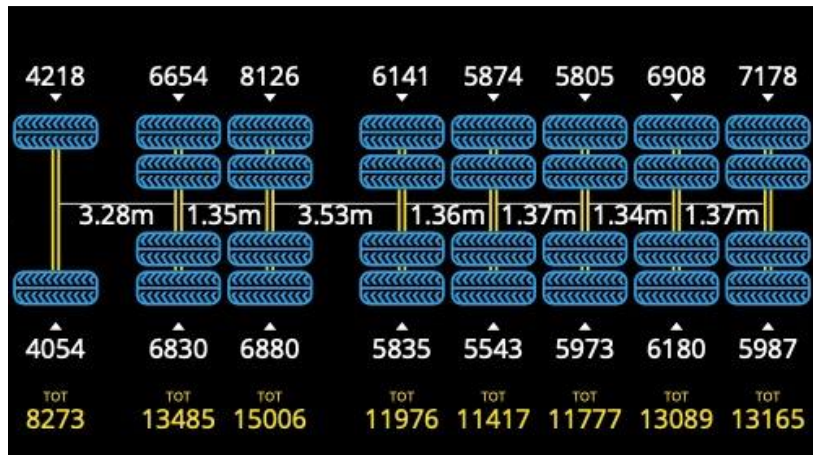


3. Methodological framework: Block 1

- **Acquires RAW data** through the **WIM** system during a monitoring period (T)
- **Pre-process data** to remove anomalies and outliers through a **Quality Control Algorithm**:



Source: IWIM Srl (2022)



```

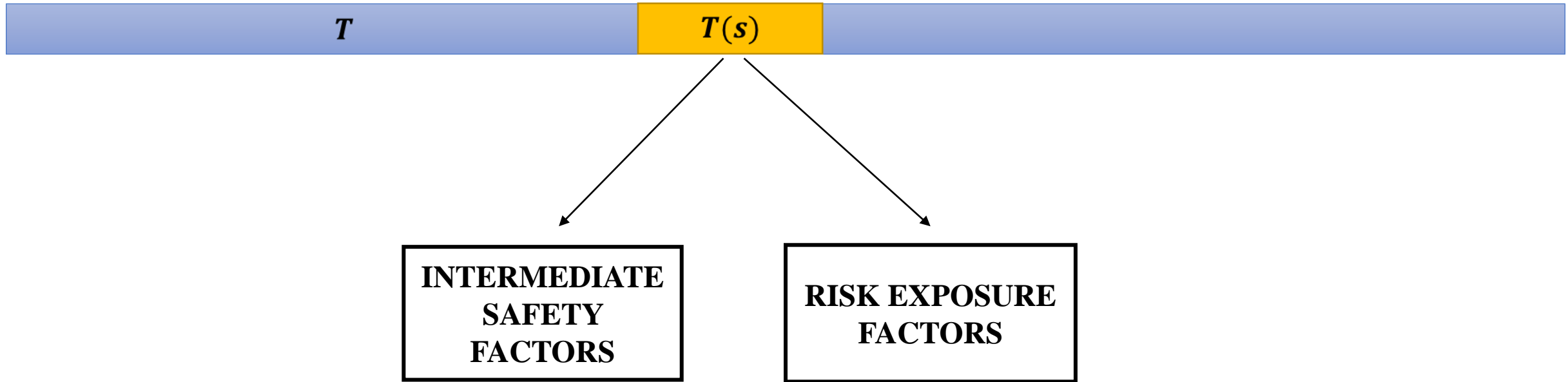
8 g=9.81;
9 crit=ones(length(data),11);
10 c=1;
11
12 %Filtro i dati al fine di eliminare le osservazioni errate ed inserisco le
13 %osservazioni corrette all'interno della struttura veh.
14 for u=1:length(data)
15     if (strcmp(data(u).outcome,'OK')||strcmp(data(u).outcome,'NL')&&strcmp(data(u).error_code,'110'))||strcmp(data(u).
16         crit(u,1)=1;
17     else crit(u,1)=0;
18     end
19     if str2num(data(u).weight)>=1000
20         crit(u,2)=1;
21     else
22         crit(u,2)=0;
23     end
24     crit(u,3)=1;
25     for j=1:str2num(data(u).axle_num)
26         if (data(u).vehicle_details.a_weight(j)>=500)&&(data(u).vehicle_details.a_weight(j)<=20000)
27             crit(u,3)=crit(u,3)*1;
28         else crit(u,3)=crit(u,3)*0;
29         end
30         if ((data(u).vehicle_details.as_weight.left(j)/data(u).vehicle_details.as_weight.right(j))>=0.5)&&((data(u).vehic
31             crit(u,4)=crit(u,4)*1;
32         else crit(u,4)=crit(u,4)*0;
33         end
34     end
35     if (str2num(data(u).length)>=2.2)&&(str2num(data(u).length)<=36)
36         crit(u,5)=1;
37     else crit(u,5)=0;
38     end

```

Source: Authors elaboration (2023)

3. Methodological framework: Block 1

- Defines equal sized **temporal slots** ($s \in S$) and computes the **intermediate safety** and **risk exposure factors** ($f_s \in F$) related to traffic load hazard acting on the bridge during each $T(s)$

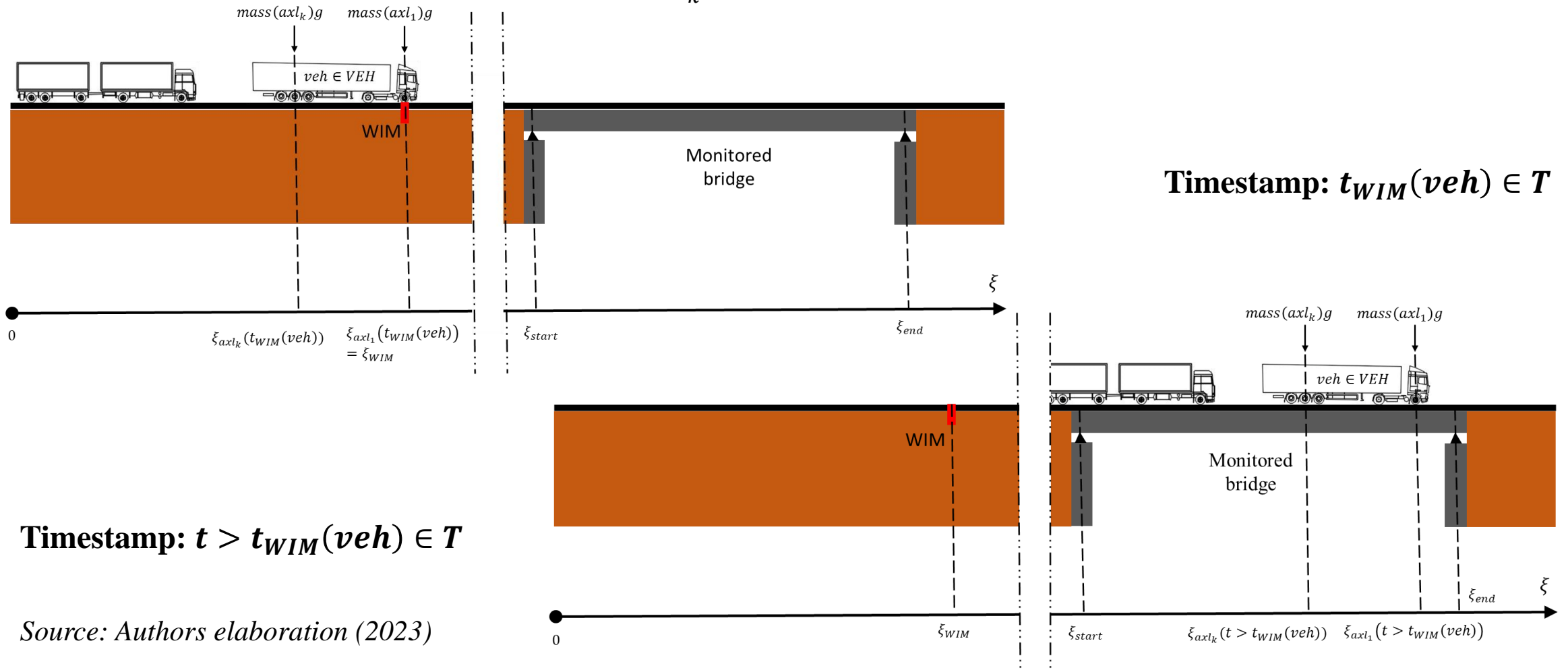


Source: Authors elaboration (2023)

$f_s \in F$

3. Methodological framework: Block 1

- Determines the **time history of the traffic load** ($G(t)$) acting on each monitored lane by analysing the **motion law** of the axles of each passing vehicle ($\xi_{axl_k}(t)$):



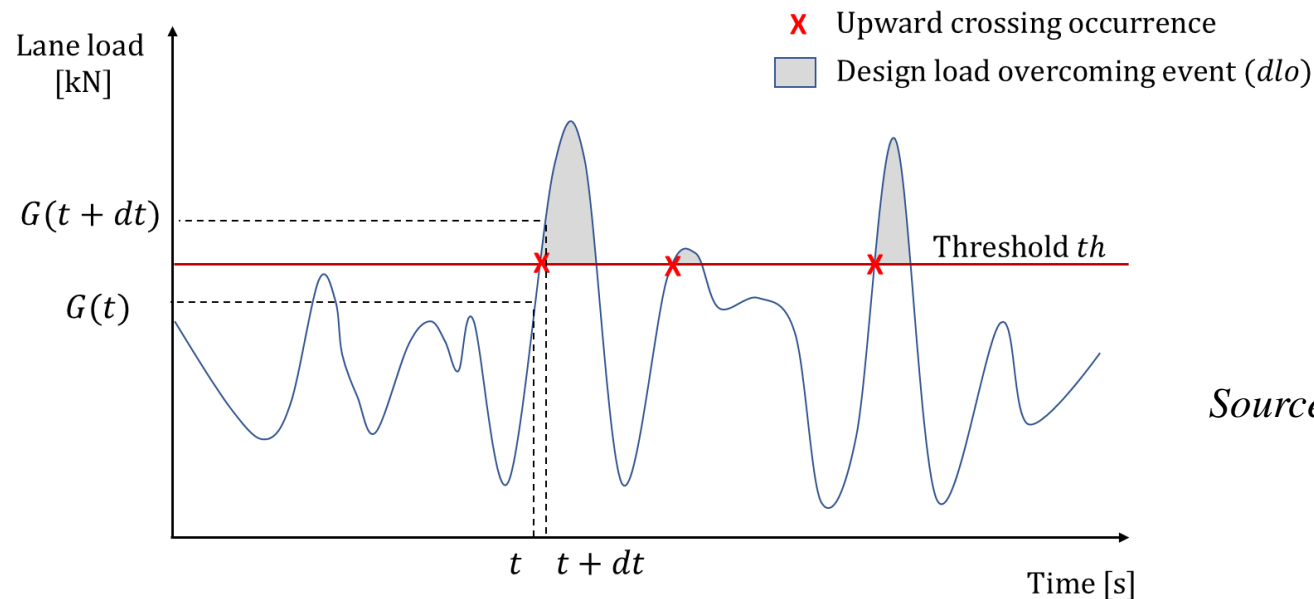
3. Methodological framework: Block 1

- Computes the **final safety outcome factors** by identifying the **design load overcoming events**, according to the lane load thresholds prescribed by **Eurocode 1** for three different Limit States.

$$th_{FuCi} \stackrel{\text{def}}{=} \max\{\gamma_Q 2Q_{ik} + \gamma_Q \psi_0 q_{ik} Lw_i; \gamma_Q q_{ik} Lw_i + \gamma_Q \psi_0 2Q_{ik}\} RF; \forall i \in I \quad (\text{ULS})$$

$$th_{ChCi} \stackrel{\text{def}}{=} \max\{2Q_{ik} + \psi_0 q_{ik} Lw_i; q_{ik} Lw_i + \psi_0 2Q_{ik}\} RF; \forall i \in I \quad (\text{ISLS})$$

$$th_{FrCi} \stackrel{\text{def}}{=} \max\{\psi_1 2Q_{ik} + \psi_2 q_{ik} Lw_i; \psi_1 q_{ik} Lw_i + \psi_2 2Q_{ik}\} RF; \forall i \in I \quad (\text{RSLS})$$



Source: Authors elaboration (2023)

3. Methodological framework: Block 2

- Defines a **risk index** related to traffic load hazard:

$$R_s \stackrel{\text{def}}{=} P_s E_s C_s = H_s(E_s, X_s) V_s(Y_s); \forall s \in S;$$

- Builds two alternative **models for predicting the frequency** of design load overcoming events:

$$\widetilde{H}_s \stackrel{\text{def}}{=} \sum_{th \in TH} |DLO(th, s)| = \alpha E_s^\beta e^{\sum_{x_j \in X} \gamma_j x_{j,s}}; \forall s \in S; \text{ (Mixed model: Power \& Negative Binomial Regression)}$$

$$\widetilde{H}_s \stackrel{\text{def}}{=} \sum_{th \in TH} |DLO(th, s)| = \tilde{\omega}(\{f_s \in F\}, \bar{\theta}_{freq,0}); \forall s \in S; \text{ (Artificial Neural Network)}$$

- Builds two alternative **models for predicting the severity** of design load overcoming events:

$$\widetilde{V}_s \stackrel{\text{def}}{=} P \left(z_s = 1 \mid \{y_{j,s}\}_{y_j \in Y} \right) = \frac{e^{\delta + \sum_{y_j \in Y} \eta_j y_{j,s}}}{1 + e^{\delta + \sum_{y_j \in Y} \eta_j y_{j,s}}}; \forall s \in S; \text{ (Binomial Logistic Regression)}$$

$$\widetilde{V}_s \stackrel{\text{def}}{=} P \left(z_s = 1 \mid \{y_{j,s}\}_{y_j \in Y} \right) = \tilde{\omega}(\{f_s \in F\}, \bar{\theta}_{sev,0}); \forall s \in S; \text{ (Artificial Neural Network)}$$






3. Methodological framework: Block 2

- **Multiplies frequency and severity predictions** to compute the risk index:

$$\widetilde{R}_s = \left(\alpha E_l^\beta e^{\sum_{j=1}^{|X|} \gamma_j x_{j,s}} \right) \left(\frac{e^{\delta + \sum_{j=1}^{|Y|} \eta_j y_{j,s}}}{1 + e^{\delta + \sum_{j=1}^{|Y|} \eta_j y_{j,s}}} \right); \forall s \in S; \text{ (Generalized Linear Regressions)}$$

$$\widetilde{R}_s = \underset{f \in F}{\widetilde{\omega}}(f_s, \bar{\theta}_{0,freq}) \underset{f \in F}{\widetilde{\omega}}(f_s, \bar{\theta}_{0,sev}); \forall s \in S; \text{ (Artificial Neural Networks)}$$

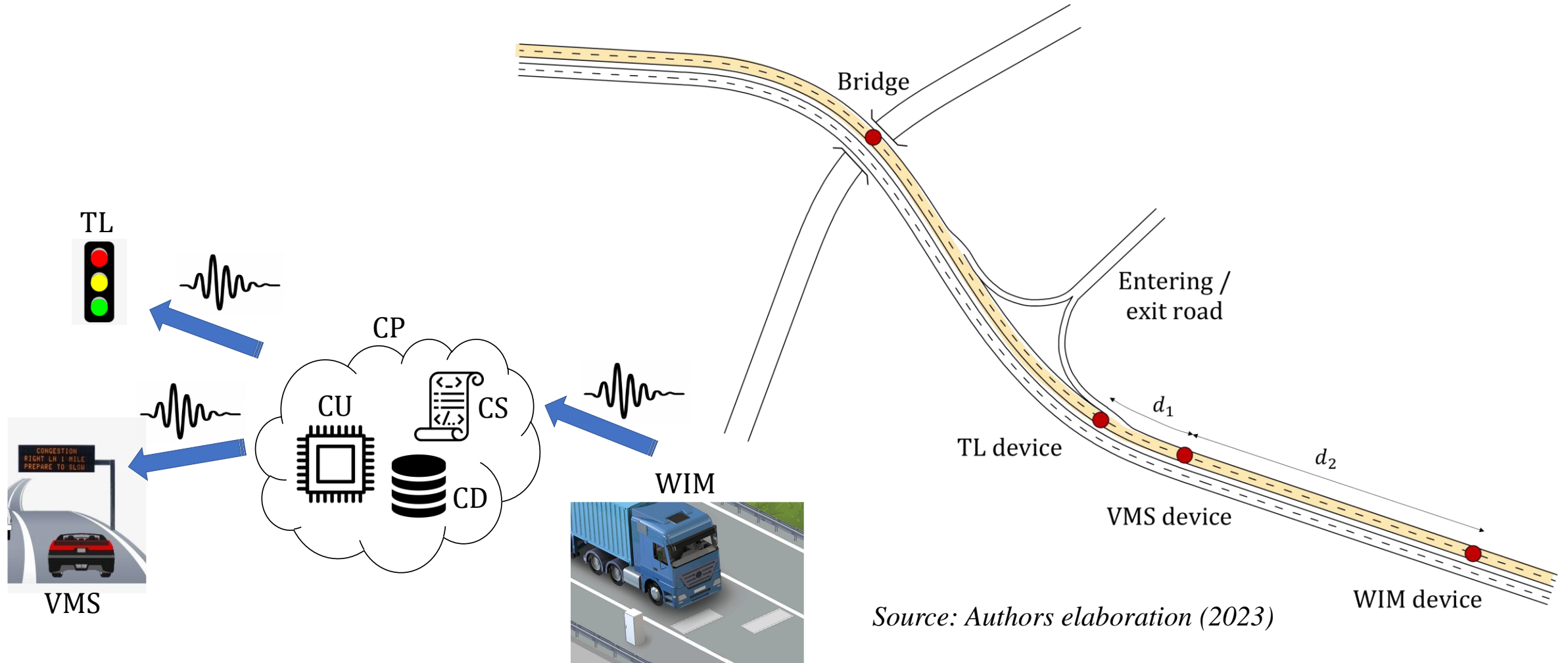
- **Defines a five-level risk scale** to rank the timeslots and identify the most critical ones:

Risk level	Lower limit	Upper limit	Colour
RL₁ - Maximum	χ_{99}	$\max R_s$	
RL₂ - High	χ_{75}	χ_{99}	
RL₃ - Above average	χ_{50}	χ_{75}	
RL₄ - Below average	χ_{25}	χ_{50}	
RL₅ - Low	$\min R_s$	χ_{25}	

Source: Authors elaboration (2023)

3. Methodological framework: Block 3

- Defines an **ITS-based risk management architecture**, composed by WIM, VMS, and TL devices.



3. Methodological framework: Block 3

- Implements a **Risk Management Algorithm** based on traffic control actions:

Algorithm Risk Management

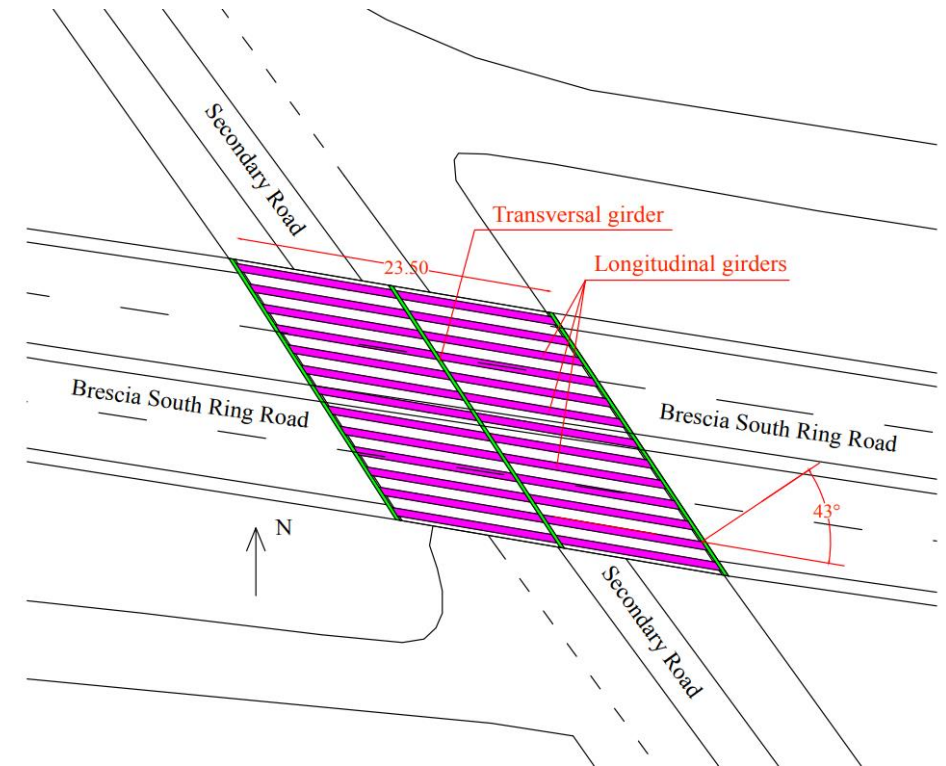
```
1 For each  $mw \in MW$ 
2   If ( $R_{mw} \in LR_1$ ) Then
3     Display "Bridge closed for all vehicles. Please take the next exit" on the VMS
4     And Trigger a yellow – red cycle on the TL
5   Else If ( $R_{mw} \in LR_2$ ) Then
6     Display "Bridge closed for vehicles with a mass over 180 t. Please take the next exit." on the VMS
7   Else Do nothing
8   End If
9 End For
```

Source: Authors elaboration (2023)

- **Periodically updates** the risk prediction model and the risk scale.

4. Real world experiment: Site

- A **bridge** along the **Brescia's South Ring Road** was considered as **Case Study**.
- It is a **simple supported** structure, composed by **13 longitudinal precast concrete girders**.









Source: Authors elaboration (2023)

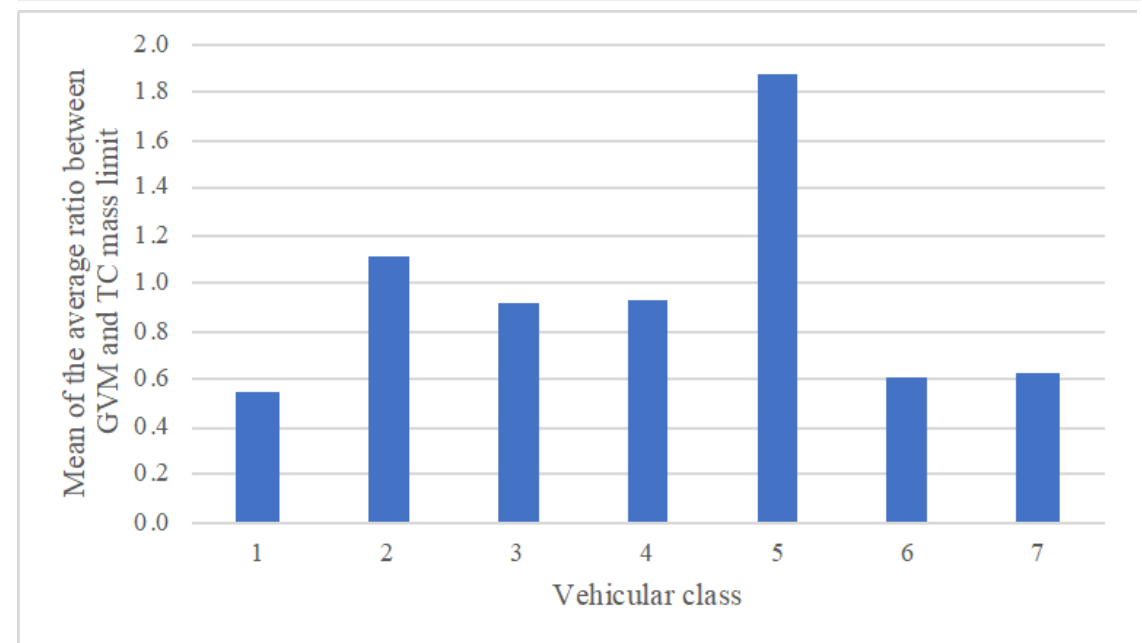
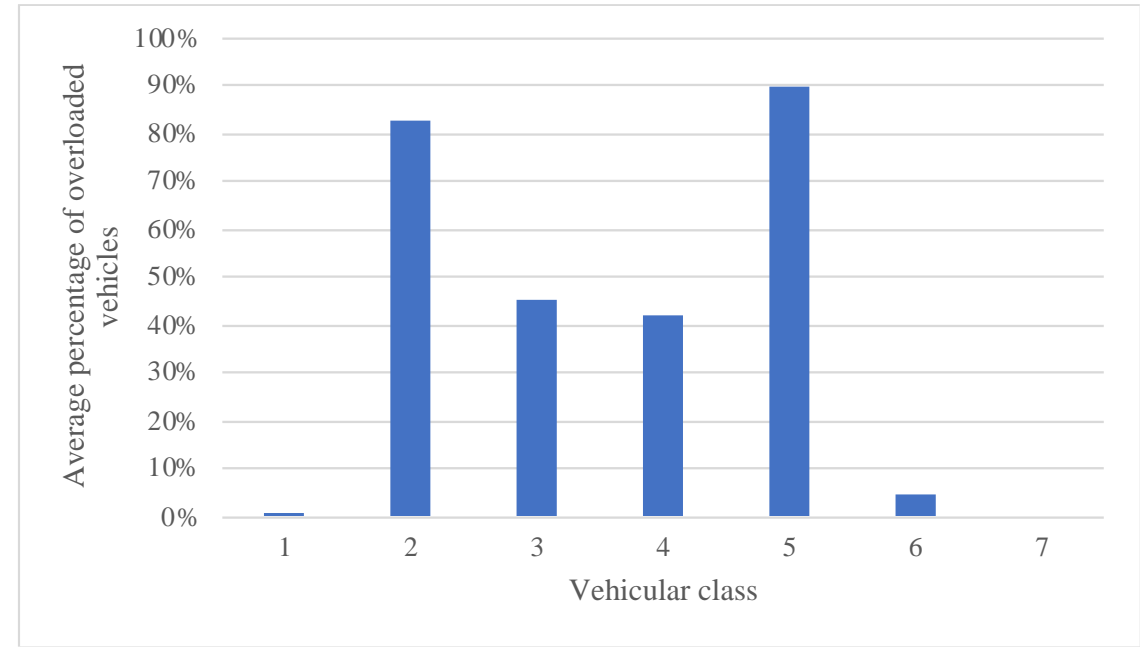
4. Real world experiment: Block 1

- **2M+ vehicles** sampled by a **WIM system** during a **five months monitoring period** were considered.

Source: Authors elaboration (2023)

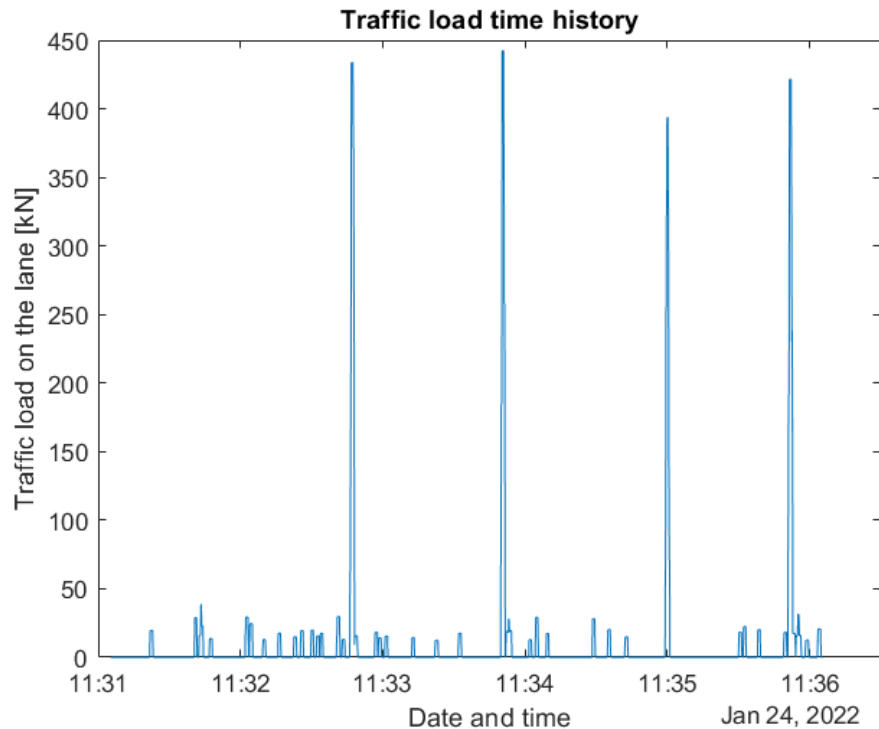
Class ID	Class name	Symbolic illustration	Fraction [%]
1	Cars and vans		92.889%
2	Single unit trucks and buses		0.597%
3	Articulated trucks up to 6 axles		5.733%
4	Road trains up to 6 axles		0.469%
5	More than 6 axles vehicles		0.309%
6	Isolated trailers		0.003%
7	Unknown vehicles	?	< 0.001%

Source: Authors elaboration (2023)



4. Real world experiment: Block 1

- The computation procedures were implemented into a **MATLAB© script**.
- The **traffic load time history** and the number of **design load overcoming events** associated to each threshold were determined.



Parameter	Symbol	Unit	Statistic	Value
Number of design load overcoming events related to FuC threshold in each $s \in S$ (ULS)	$ DLO(th_{FuC1}, s) $	-	Mean	0.000
			Standard dev.	0.000
			Minimum	0
			Maximum	0
			Sum	0
Number of design load overcoming events related to ChC threshold in each $s \in S$ (ISLS)	$ DLO(th_{ChC1}, s) $	-	Mean	0.005
			Standard dev.	0.075
			Minimum	0
			Maximum	2
			Sum	14
Number of design load overcoming events related to FrC threshold in each $s \in S$ (RSLs)	$ DLO(th_{FrC1}, s) $	-	Mean	5.298
			Standard dev.	8.056
			Minimum	0
			Maximum	57
			Sum	15'132

Source: Authors elaboration (2023)

- The **Generalized Linear Regressions** models for predicting the two risk components were determined:

Item	Description	Est.	p-val	Item	Description	Est.	p-val
$\log(\alpha)$	Natural logarithm of the constant	-1.920	0.516	γ_8	Mean interaxle	1.084	0.085
β	Exponent of the exposure factor	0.400	<.001	γ_9	Maximum GVM length ratio	0.00043	<.001
γ_1	Weekend (wrt weekday)	-0.510	<.001	γ_{10}	Mean GVM overload ratio – Class 1 (Cars and vans)	3.910	0.005
γ_2	Class 2 (Single unit trucks and buses) fraction	-8.390	0.158	γ_{11}	Maximum GVM overload ratio – Class 2 (Single unit trucks and buses)	-0.151	0.111
γ_3	Class 5 fraction (More than 6 axles vehicles)	6.950	0.242	γ_{12}	Maximum GVM overload ratio – Class 4 (Road trains up to 6 axles)	0.403	<.001
γ_4	Mean speed	-0.042	<.001	γ_{13}	Overloaded vehicles fraction – Class 3 (Articulated trucks up to 6 axles)	1.711	<.001
γ_5	Minimum length	-2.094	0.017	γ_{14}	Overloaded vehicles fraction – Class 4 (Road trains up to 6 axles)	0.271	0.018
γ_6	Mean axle imbalance ratio	-1.426	0.011	γ_{15}	Extremely loaded vehicles following one another	0.150	0.043
γ_7	Maximum axle imbalance ratio	0.438	0.045	γ_{16}	Overloaded axles fraction	19.260	<.001

Summary statistics

Source	Degree of freedom	Deviance	Mean deviance	Parameter	Value
Regression	17	4'303.1	253.13	d.r.	253.13
Residual	2'139	569.5	0.266	χ^2	.001
Total	2'156	4'872.7	2.260		

Frequency model (GLR)

Source: Authors elaboration (2023)

Compliance with Traffic Code prescriptions are strong predictors!

Severity model (GLR)

Source: Authors elaboration (2023)

Item	Description	Estim.	OR	p-val.	Item	Description	Estim.	OR	p-val.
δ	Constant	-188.9		0.027	η_4	Minimum axle imbalance ratio	-38.7	1.56E-17	0.029
η_1	Maximum length	0.464	1.590	0.061	η_5	Maximum GVM overload ratio	55.8	1.71E+24	0.019
η_2	Maximum axle mass	0.00212	1.002	0.091	η_6	Maximum GVM overload ratio – Class 1 (Cars and vans)	-23.48	6.35E-11	0.018
η_3	Mean axle imbalance ratio	50.4	7.73E+21	0.091					

Summary statistics

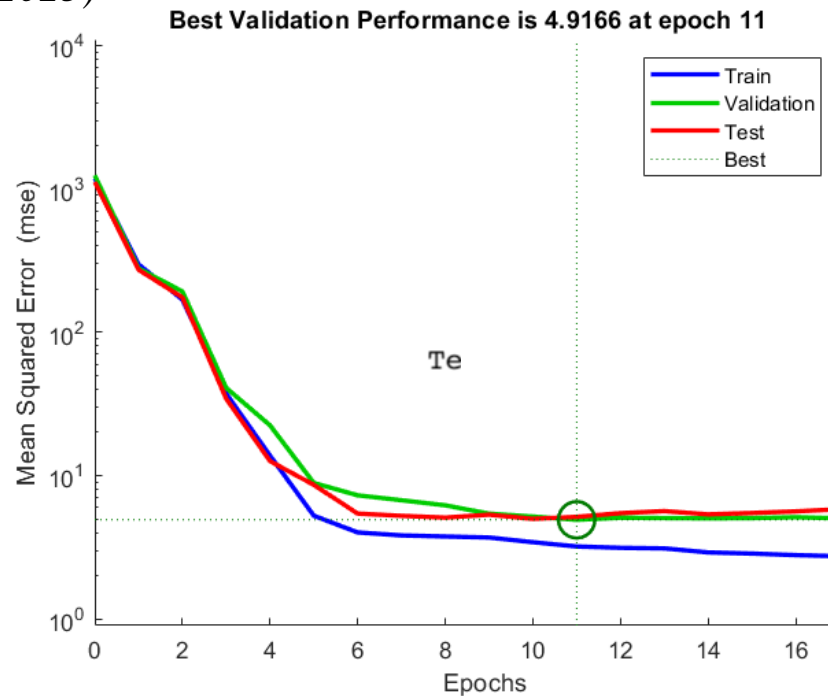
Source	Degree of freedom	Deviance	Mean deviance	Parameter	Value
Regression	6	101.02	16.836	d.r.	16.84
Residual	1'224	15.16	0.012	χ^2	<.001
Total	1'230	116.18	0.094		

4. Real world experiment: Block 2

- The **Artificial Neural Networks** models for predicting the two risk components were determined:

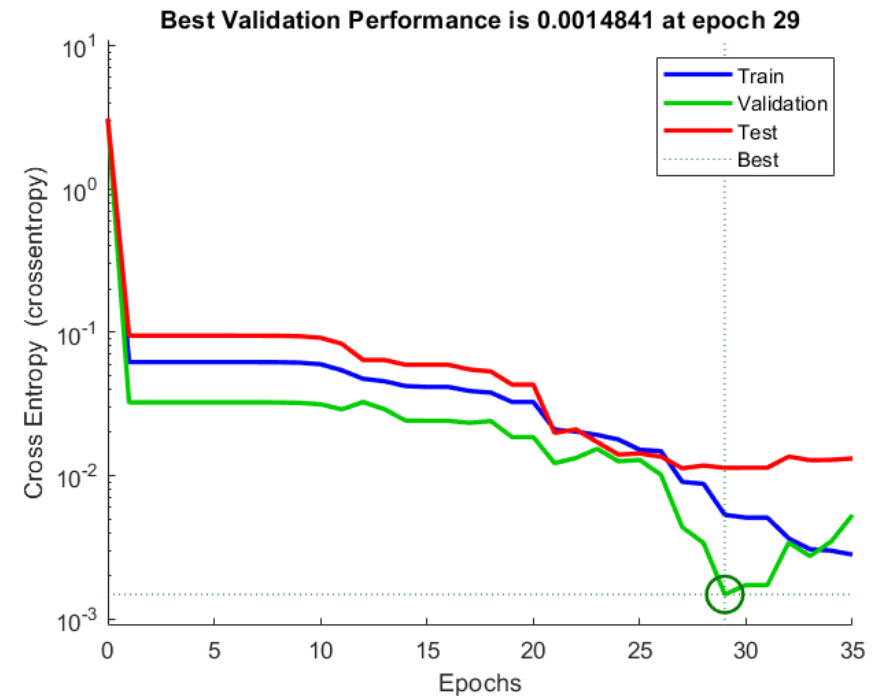
Source: Authors elaboration (2023)

Frequency model



- 2 layers feed-forward**
- 10 perceptrons** in hidden layer
- Sigmoidal** activation functions
- Levenberg-Marquardt** training algorithm

Severity model



- 2 layers feed-forward**
- 10 perceptrons** in the hidden layer
- Sigmoidal** activation functions
- Scaled Conjugate Gradient** training algorithm

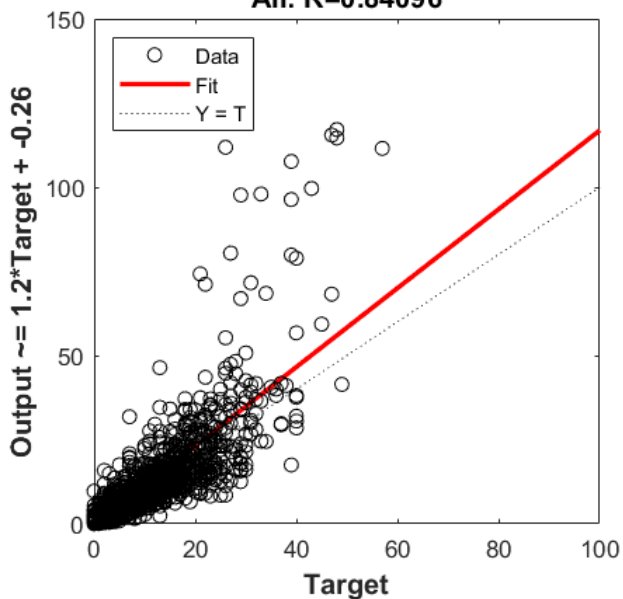
4. Real world experiment: Block 2

- The performances of the alternative models were compared:

Model	R (train)	R (test)	R (all)	π	\mathcal{S} (train)	\mathcal{S} (test)	\mathcal{S} (all)	CE
Freq - GLR	0.838	0.859	0.841	1.120	na	na	na	na
Freq - ANN	0.977	0.961	0.972	0.999	na	na	na	na
Sev - GLR	na	na	na	na	80.0%	66.7%	76.9%	0.0077
Sev - ANN	na	na	na	na	88.9%	100.0%	92.3%	0.0034

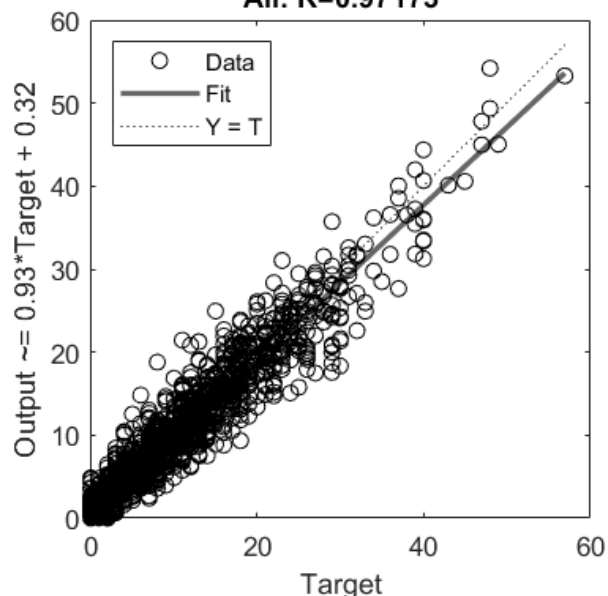
Freq - GLR

All: $R=0.84096$



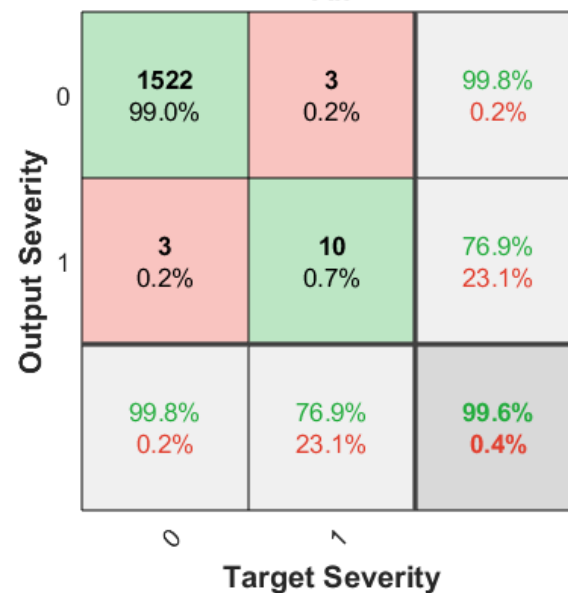
Freq - ANN

All: $R=0.97173$



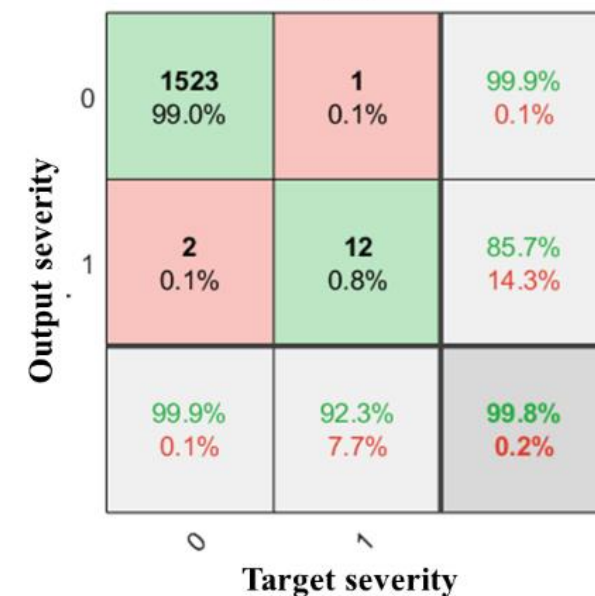
Sev - GLR

All



Sev - ANN

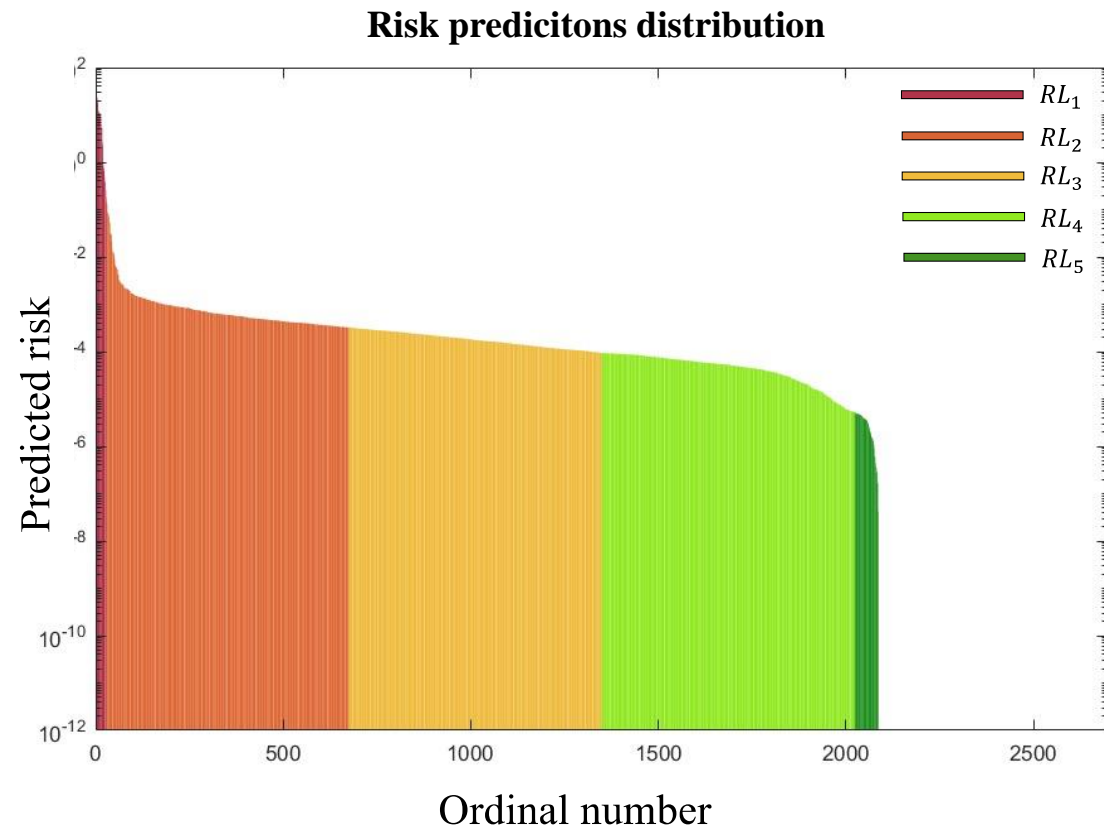
All



Source: Authors elaboration (2023)

4. Real world experiment: Block 2

- The **timeslots** were **classified** according to the predicted risk by adopting the **five-level risk scale**:



Source: Authors elaboration (2023)

Timeslots weekly distribution

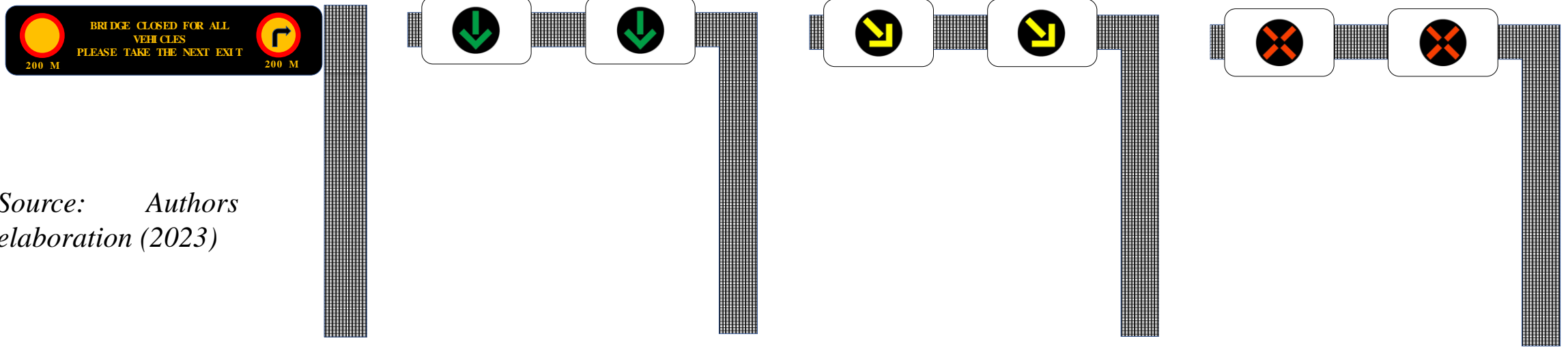
Day of the week	RL ₁	RL ₂	RL ₃	RL ₄	RL ₅
1	0.00%	0.46%	1.34%	19.44%	33.23%
2	3.70%	21.33%	16.47%	10.98%	8.01%
3	14.81%	19.47%	18.84%	12.61%	7.86%
4	37.04%	20.56%	20.18%	12.02%	7.12%
5	22.22%	17.00%	17.21%	12.02%	9.20%
6	22.22%	19.01%	17.06%	10.53%	10.53%
7	0.00%	2.16%	8.90%	22.40%	24.04%
Total	100.00%	100.00%	100.00%	100.00%	100.00%

Timeslots daily distribution

Hour	RL ₁	RL ₂	RL ₃	RL ₄	RL ₅
0	0.00%	0.46%	1.48%	3.71%	10.68%
1	3.70%	0.62%	1.04%	1.63%	12.91%
2	0.00%	2.78%	2.67%	3.26%	7.57%
3	0.00%	4.33%	2.97%	2.52%	7.27%
4	0.00%	9.89%	2.97%	1.78%	2.67%
5	0.00%	11.28%	2.97%	0.45%	2.67%
6	0.00%	10.82%	3.71%	1.04%	1.78%
7	7.41%	1.39%	6.08%	6.97%	2.23%
8	0.00%	1.70%	6.23%	7.42%	1.93%
9	7.41%	4.02%	7.57%	3.71%	1.63%
10	3.70%	7.57%	4.75%	3.12%	1.78%
11	18.52%	7.57%	4.30%	1.93%	2.52%
12	14.81%	8.04%	4.15%	3.26%	1.34%
13	11.11%	8.04%	4.45%	3.26%	1.19%
14	7.41%	5.72%	5.93%	3.41%	1.78%
15	18.52%	3.40%	6.08%	4.30%	2.37%
16	7.41%	4.64%	5.93%	3.12%	2.82%
17	0.00%	0.77%	4.90%	8.61%	2.37%
18	0.00%	2.78%	5.93%	4.75%	3.12%
19	0.00%	1.70%	4.75%	7.57%	2.23%
20	0.00%	0.31%	4.45%	8.46%	2.97%
21	0.00%	0.77%	2.67%	7.42%	5.34%
22	0.00%	0.62%	2.08%	5.49%	8.01%
23	0.00%	0.77%	1.93%	2.82%	10.83%
Total	100.00%	100.00%	100.00%	100.00%	100.00%

4. Real world experiment: Block 3

- The layout of the ITS architecture was designed and the traffic management actions were simulated:



Source: Authors elaboration (2023)

Risk management actions	Number of involved monitoring windows	Percentage of involved monitoring windows	Involved vehicular flow	Percentage of involved vehicular flow
Display "All vehicles must take the next exit" on the VMS				
AND	6'923	1.34 %	7'354	3.69 %
Trigger a yellow-red cycle on the TL				
Display "All vehicles heavier than 108t must take the next exit" on the VMS	53'155	10.25 %	22'500	11.28 %
Do nothing	458'303	88.41 %	169'621	85.03 %

5. Discussion

The **main findings** can be summarized in the following points:

- WIM data revealed a **high percentage of overloaded vehicles** on the bridge.
- Bridge design loads were exceeded with a **return period significantly shorter** than that prescribed by **Eurocode 1** ($T_r \cong 8 \text{ days} \ll 1'000 \text{ years}$ for ChC).
- **ANNs outperformed GLRs** in predicting both frequency and severity components.
- Safety factors related to the **compliance with TC mass limits prescriptions** showed a high influence on risk predictions.
- Approximately **4% passing flow** would be interested by the **more severe traffic management actions**.

6. Conclusions

The findings have at least **three practical consequences**:

- Recommend **enforcement strategies** for the **identification** and **sanctioning** of **illegal overloaded vehicles** that travel on the bridge without any authorization.
- Suggest a **greater caution** by the RAs when **permits for extremely overloaded vehicles** are issued.
- Recommend a **widespread deployment of ITS-based architectures** for the real time management of the risk posed by the traffic load hazard.

6. Conclusions

This study indicates several **future developments**:

- Considering **traffic load effects** instead of traffic load itself. Indeed, for the same total load, the internal actions can be very dissimilar for **different load configurations**.
- Integrating **new variables** based on data acquired by **other sensors** (e.g., accelerometers, strain gauges, intelligent traffic cameras, etc.).
- Prioritising **traffic management actions at network level**, taking into account the negative consequences of traffic interruptions.

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*Department Civil, Environmental, Architectural
Engineering and Mathematics (DICATAM)*

Roberto VENTURA

Post-Doctoral Researcher,
University of Brescia, Italy
r.ventura001@unibs.it

Benedetto BARABINO

Associate Professor,
University of Brescia, Italy
benedetto.barabino@unibs.it

Giulio MATERNINI

Full Professor, University of Brescia, Italy
University of Brescia, Italy
giulio.maternini@unibs.it

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